

Journal of Mining & Environment, Vol.8, No.2, 2017, 291-304. DOI: 10.22044/jme.2016.654

# Improved prediction of blast-induced vibrations in limestone mines using Genetic Algorithm

M. Ataei<sup>\*</sup> and F. Sereshki

School of Mining, Petroleum & Geophysics Engineering, Shahrood University of Technology, Shahrood, Iran

Received 2 May 2016; received in revised form 20 June 2016; accepted 27 June 2016 \*Corresponding author: ataei@shahroodut.ac.ir (M. Ataei).

### Abstract

Like most limestone mines, which produce the raw materials required for cement companies, the transportation cost of the raw materials used in the Shahrood Cement Company is high. It has been tried to build the crushing and grinding plant close to the mine as much as possible. On the other hand, blasting has harmful effects, and the impacts of blast-induced damages on the sensitive machinery, equipment, and buildings are considerable. In such mines, among the blasting effects, blast-induced vibrations have a great deal of importance. This research work was conducted to analyze the blasting effects, and to propose a valid and reliable formula to predict the blast-induced vibration impacts in such regions, especially for the Shahrood Cement Company. Up to the present time, different indices have been introduced to quantify the blast vibration effects, among which peak particle velocity (PPV) has been widely considered by a majority of researchers. In order to establish a relationship between PPV and the blast site properties, different formulas have been proposed till now, and their frequently-used versions have been employed in the general form of  $PPV = K_1 W^{K_2} D^{K_3}$ , where W and D are the maximum charge per delay and the distance from the blast site, respectively, and  $K_1, K_2$ , and  $K_3$  describe the site specifications. In this work, a series of tests and field measurements were carried out, and the required parameters were collected. Then in order to generalize the relationship between different limestone mines, and also to increase the prediction precision, the related data for similar limestone mines was gathered from the literature. In order to find the best equation fitting the real data, a simple regression model with genetic algorithm was used, and the best PPV predictor was achieved. At last, the results obtained for the best predictor model were compared with the real measured

Keywords: Blasting, Blast-Induced Vibration, PPV, Limestone Mine, Cement Company, Genetic Algorithm.

## 1. Introduction

Companies using blasting operations are often faced with the necessity of limiting the vibration levels in order to minimize or eliminate the possibility of damage to the nearby structures. Therefore, proper blasting design is necessary to ensure both the safety of employees and the protection of nearby structures from the vibration effects [1]. Generally, a blasting project has four forms of concerns including fly rock, air blast, produced dust and fume, and vibration. The importance of each item depends on studying the

data by means of a correlation analysis.

case conditions and environmental aspects. In the case of the Shahrood Cement Company, due to the material transportation costs, it has been tried to make the factory and crushing plant close to the limestone mines as much as possible, and therefore, the blasting effects, mostly the vibrations, play the most important role in a blast design. An overview of the Shahrood Cement Company and one of its limestone mines is shown in Figure 1.



Figure 1. An overview of Shahrood Cement Company and one of its limestone mines.

The intensity of ground vibrations depends on various parameters. These parameters can be broadly divided into two categories, namely the controllable and uncontrollable parameters, as shown in Table 1. The controllable parameters can be changed by the engineers in charge, while the uncontrollable ones are natural and uncontrollable [2]. In order to establish a vibration predictor model, taking all these parameters in a single relation into account is impractical, and such an equation could not be used easily in practical situations. Engineering experiences show that the vibration level at any particular site is affected mostly by the maximum charge weight per delay (W). distance from the blast site (D), vibration frequency (f), and initiation method [3]. Among them, the vibration frequency is uncontrollable, and the initiation method is of less importance, and therefore, the analysis should be focused on the two controllable parameters (i.e. D and W). To simplify the relation between the blast-induced vibration impact and the blast and site parameters, two important enterprises should be attempted, as explained below.

First, to estimate the blast impacts and effects on the environment, buildings, and structures, an

index or indicator should be defined, and this index should be a proper representative of the blast vibration impact, and yet, easy to use and measure. Actually, to estimate the damage level in the structures produced by blasting projects, a lot of studies have been conducted, and to determine the damage level, some parameters such as peak particle velocity (PPV), peak particle acceleration (PPA), peak particle displacement (PPD), scaled distance (SD), and energy ratio (ER) have been investigated. For estimating the damage of blast vibrations, the importance of different parameters has been considered by different sources and researchers, given in Table 2. For example, the US Bureau of Mines (USBM) has extensively studied the various aspects of the ground vibrations caused due to the open-cast blasting and damaging effects on different types of structures. They have found that PPV is the best index for use to determine the damage criteria for the structures [3]. Table 1 shows that, from the viewpoint of almost all researchers and sources, PPV is the best index for use to evaluate the blasting vibration effects, and so in this study, PPV was used as the vibration impact estimating the index or indicator.

	Controllable vari	ables		Uncontrollable variables
Geometrical parameters	Explosive- dependent parameters	<b>Operational</b> parameters	Others	Delay time scatter
Hole diameter Burden Spacing Bench height Stemming Hole inclination Sub-drilling	Explosive type Total explosives Max. Charge/delay Explosive energy VOD P-wave in rock	Blast size Initiation point Delay sequence Delay intervals Firing method Confinement	Distance to object	Rock conditions Topography Geology Rock properties Weather Conditions

Table1. Controllable and uncontrollable parameters affecting vibration intensity [2].

Parameters	1	2	3	4	5	6	7	8	9	10	11	12	13	14
Particle velocity				٠	•	•		•	٠	•	٠	•	•	•
Frequency		•	•				•		•		•	•	•	
Damage type				٠	•	•				•	•			
Structure type										•	•	•	•	
Rock type								•						
Acceleration			•				•							
Amplitude	٠	٠												
Distance		•												•
Vibration velocity							•							
Displacement							•							
1. United States Bureau	ı of M	ines	(USB	M),19	942 []	[6]	8. I	ndian	Stan	dard In	stitute	, 1973	[22]	
2. Rockwell's Energy I	Formu	la, 19	34 [1	7]			9. Medearis's Approach, 1976 [23]							
3. Crandell's Energy R	atio C	oncep	ot, 19	49 [1	8]		10.	Cann	net, B	auer a	nd Cal	der, 19	77 [24	]
4. Langefors, Kihlstrom and Westerberg, 1957 [19]						11. USBM's Criterion, 1980 [25]								
5. Edwards and Northweed, 1959 [20] 12. German DIN Standard 4150, 1986 [26]							26]							
6. USBM's Criterion, 1971 [21] 13. Indian CMRI standards, 1993 [27]														
7. Langefors and Kihls	trom's	Cha	rt, 19	67 [6]	]		14.	Rose	nthal	and M	orlock	, 1987	[28]	

Table 2. Importance of different parameters used for estimating damage of blast vibrations [10-15].

Secondly, a proper and reliable relation should be employed between the distance from the blast site (D) and the maximum charge per delay (W) as the most important controllable and effective parameters for the PPV intensity, and yet, it should be easy to use and analyze. During years, in different parts of the world, a lot of projects have been conducted to develop a suitable relationship between PPV, D, and W, and many scientists and engineers have investigated the PPV prediction and published their findings. The first significant PPV predictor equation was proposed by USBM [4]. There are also some modified predictors suggested by other researchers or institutions such as Langefors and Kihlstrom [5], Ambraseys and Hendron [6], Ghosh and Daemen [7], Roy [8], and Singh et al. [9]. However, the

PPV predictor established by USBM is still the most widely used equation in the literature. In order to analyze the vibration data, some frequently-used PPV predictor models have been listed in Table 3. All these formulas were used to predict PPV and analyze the Shahrood Cement Company measured data, and in addition, in order to generalize these relations for a wide range of limestone mines, and also to increase the prediction precision, some related data from similar limestone mines was gathered from the literature. Next, to find the best fitted equation to the real data, a simple regression model associated with genetic algorithm (GA) optimization method was used, and the best PPV predictor was achieved.

	Table 3. List of proposed predictor equations used for calculation of PPV.								
Model	Name	Year	Equation	Reference					
1	USBM(Duvall and Fogelson, 1962)	1962	$PPV = K_1 \left(\frac{D}{\sqrt{W_{\text{max}}}}\right)^{-K_2}$	[4]					
2	Langfors & Kihlstrom(1968)	1968	$PPV = K_1 \left( \sqrt{\frac{W_{\text{max}}}{D^{2/3}}} \right)^{K_2}$	[5]					
3	Ambrases & Hendron(1968)	1968	$PPV = K_1 \left(\frac{D}{\sqrt[3]{W_{\text{max}}}}\right)^{-K_2}$	[6]					
4	Indian standard predictor(1973)	1973	$PPV = K_1 \left(\frac{W_{\text{max}}}{D^{2/3}}\right)^{K_2}$	[22]					

Table 3. List of proposed predictor equations used for calculation of PPV.

### 2. Instrumentation and data measurement

Blast-induced vibrations were monitored by a Minimate Plus seismograph (made by M/s Instantel Inc). This seismograph has four

channels, three of which are allocated to the vibration measurement in three directions, i.e. longitudinal (Lon.), vertical (Ver.), transverse

(Tran.), and the fourth one, which is an air phone, measures the air blast. This seismograph also records the dominant vibration frequency, peak particle acceleration (PPA), and peak particle displacement (PPD), and computes the peak vector sum (PVS) of vibration. PVS represents the resultant particle velocity magnitude, and is defined as follows:

$$PVS = \sqrt{V_L^2 + V_T^2 + V_V^2}$$
(1)

Where  $V_L$ ,  $V_T$ , and  $V_V$  are the longitudinal, transverse, and vertical components of vibration, respectively. The setup of the Minimate Plus seismograph in the Shahrood Cement Company is given in Figure 2.

The distances from the blasting site to the monitoring stations were measured precisely by

means of a hand-held global positioning system (GPS) instrument, and the amount of charge weight per delay was recorded for each shot by controlling the hole charges. In determining the maximum charge per delay, the amount of dynamite used as priming was added to the amount of blasting agent. The required blasting design parameters and the measurement results are given in Table 4. In the blasting operations, ANFO (blasting agent) and gelatin dynamite (priming) were used as the explosives during the study. The blast holes were vertical and 64 mm in diameter. The holes length varied from 3.3 to 6m, with approximately 0.5 m of sub-drillings and one-third of hole length as stemming for all blast patterns. An electrical millisecond delay system was used to initiate the charge.



Figure2. Setup of Minimate Plus seismograph.

Table 4. Measured vibration data in Shahrood Cement Company limestone mines.									
	Shot1	Shot 2	Shot 3	Shot 4	Shot 5	Shot 7			
W (kg)	945	665	810	810	270	63			
<b>D</b> (m)	667.41	771.94	785.45	334.75	451.56	113.93			
Tran (mm/s)	0.381(228Hz)	1.27 (5.4Hz)	1.40(15.3 Hz)	0.762 (11.5Hz)	0.508 (8.7 Hz)	6.22 (24.7 Hz)			
Ver (mm/s)	0.508(205Hz)	0.762(4.0 Hz)	1.40(66 Hz)	0.508(10.6Hz)	0.381(15.6 Hz)	8.51(24.4 Hz)			
Lon (mm/s)	0.508(128Hz)	0.762(3.7 Hz)	2.03(26.9 Hz)	0.762(26.3Hz)	0.508(26.3Hz)	10.0(30.1 Hz)			
PPV (mm/s)	0.81	1.67	2.82	1.19	0.81	14.53			
Mic (Pa)	< 0.500	39.5(22Hz)	462(22.3 Hz)	50.3(15.8Hz)	52.0(49 Hz)	301(54 Hz)			
PDT (mm)	0.0031	0.0407	0.0122	0.0116	0.00944	0.0384			
PDV (mm)	0.00045	0.0266	0.00792	0.00778	0.00429	0.0559			
PDL (mm)	0.00071	0.036	0.0146	0.011	0.0033	0.052			
PAT(g)	0.053	0.053	0.106	0.053	0.053	0.212			
PAV (g)	0.106	0.106	0.106	0.053	0.053	0.212			
PAL(g)	0.053	0.053	0.106	0.053	0.106	0.212			
PVS (mm/s)	0.762	1.28	2.05	1.09	0.568	11.5			

# **3.** Frequency analysis of blast vibration

Using the Minimate Plus seismograph, the dominant frequency and also the frequency range between 2 Hz and250 Hz were measured for each blasting sequence. The different frequency classifications based on the USBM standards are given in Table 5, and the recorded frequencies of blasting were classified in Figure 3 based on the USBM standards.

Since the same-value vibrations with different frequencies have different impacts on buildings and structures, a Fast Fourier Transform (FFT) analysis was used in this research work. In simple waveforms, which are not composed of many different frequencies, the dominant frequency may be at the peak particle velocity. In more complex waveforms, the dominant frequency is not necessarily the frequency at the peak particle velocity but at the frequency with the greatest amplitude. Whether or not this frequency actually contributed to the peak particle velocity value, it should be found by extrapolation of the closed original signal. The frequency at the peak of a complex wave is usually not a single frequency: rather it is a series of waves of different frequencies superimposed. It is intended that the frequency spectrum data be used as a tool in conjunction with the velocity versus time waveform. Burrus and Parks [29], Light hill [30], Oppenheim, [31], Rabinerand Gold [32], and Brigham [33] have widely worked on the FFT analysis.

In fact, the frequency of blast-induced waves is generally controlled by geological conditions and delay arrangements. There are geological forms and structures that are favorable to the formation of different types of frequency waves. When the incoming vibration has a frequency in the range of natural frequency of the structure, resonance occurs and the resultant amplitude of vibration on the structure is amplified [28]. Figure 4 shows the blast time history recorded in the Shahrood Cement Company. Figure 5 is the Fast Fourier Transform (FFT) analysis of the same blast event. It is evident from Figures 4 and 5 that the duration of the blast wave is very short, and that the dominant frequency of all channels falls into the 16-27 Hz range. In fact, the damage potentials in the low frequency range (<40 Hz) are considerably higher than those in the high frequency range (>40 Hz), especially due to a possible resonance effect in structures [35, 36].

Thus one of the important parameters involved in the damage of different frequencies is related to the closeness of the blast dominant frequency and natural frequency of the structures. Using the FFT diagrams, the blasting frequency distribution can be analyzed and also compared with the natural frequency of the structures, and then the damage level can be estimated properly.

In different standards for different frequency ranges, the acceptable levels of vibration velocities are different. For example, for a typical blast vibration time history and an FFT analysis, shown in Figures 4 and 5, the acceptable level of the blast-induced vibration velocities based on USBM RI8507, OSMRE, and CMRI, the proposed Indian standards are shown in Figure 6. It is obvious that the frequency ranges and the acceptable levels of vibration velocities are different in each standard.

As an individual wave passes through the ground, high frequencies will be attenuated, and just low frequencies would remain. A curve was estimated between the dominant frequency and distance of blast site, given in Figure 7. Instead of having a descending trend, this curve has an ascending trend because it is not for a single blasting sequence but in which the blast parameters such as the charge per delay vary for each shot.

Table 5.	Different frequency	y classific	ations bas	ed on USB	M standa	rds [41].
	Frequency range	0-4 Hz	5-12 Hz	12-40Hz	>40 Hz	-
_	Description	Lowest	Low	Medium	High	_
	28.57%	7 149	50%	□0 ■5 □1. ■>	To 4 Hz To 1 2 Hz 3 To 40 Hz 40 Hz	

Figure 3. Frequency distribution resulting from Shahrood Cement Company blasting based on USBM standards.



Time Scale: 0.50 sec/div | Amplitude Scale: Geo: 5.00 mm/s/div Mic: 100.0 pa.(L)/div Figure 4.Typical blast vibration time history recorded in Shahrood Cement Company by MinimatePlus seismograph (T6.22 mm/s, F24.7 Hz; V8.51 mm/s, F24.4 Hz; L10.0 mm/s, F30.1 Hz).



(dominant frequencies for Mic, T, V, and L are 16.4, 25.3, 16.4 and 26.8 Hz, respectively).

### CMRI proposed Indian Standards



Figure 6. Acceptable level of blast-induced vibration velocities for typical FFT analysis shown in Figure 5 based on OSMRE and CMRI proposed Indian standards [22].



Figure 7. Frequency vs. distance from blast site.

### 4. Analysis of recorded data for Shahrood Cement Company

The vibration hazards of the sensitive machinery and buildings such as kiln, high buildings, fans, and grinders in the studied area have become an important concern. For this reason, a series of measurements were made in this field, and the required data was tabulate in Table 4.

Two important results can be extracted from Table 4, as follow.

First, PPA and PPD could not be such proper indicators to estimate the vibration damage level because their variations are not considerable in different locations.

Secondly, some researchers have included frequency along with PPV for the damage criteria [25, 37-39]. As it can be seen in their works, in addition to PPV, vibration frequency is an important index to indicate the damage level because, under approximately the same conditions, the vibration frequency is not the same

in different directions (i.e. longitudinal, vertical, and transverse), and also a considerable amount of energy is transported by vibration, which has a low frequency [40].

After all, the predictors listed in Table 3 were used to analyze the data in Table 4 and the related curves given in Figure 8. The statistical summary of each model was brought in Table 6. The results of this table show that among of these models, the Langfors & Kihlstrom (1968) and Indian standard (1973) predictors, are comparatively suitable but not satisfactory and reliable. For this reason, and also to generalize the relationship between similar limestone mines, and also in order to increase the prediction precision, some related data for similar mines were gathered from several references, and the same analysis was followed.



Figure 8. PPV prediction based on different predictors listed in Table 3 for recorded data in Shahrood Cement Company. a) USBM, b) Langfors & Kihlstrom (1968), c) Ambrases & Hendron (1968), and d) Indian standards.

Table 6. Statistical summary of PPV predictor models for Shahrood Cement Company data.

Tuble of Studistical Summary of TT + predictor models for Shamfood Cement Company auta							
Predictor model	USBM	Langfors & Kihlstrom(1968)	Ambrases & Hendron(1968)	Indian standard			
$R^2$	0.10	0.41	0.29	0.41			
$k_1$	18.30	9.25	194.62	9.25			
$k_2$	-0.92	-2.11	-1.28	-1.05			

# 5. Establishing a general model for limestone mines

In order to establish a general model for limestone mines, a series of recorded data were gathered from different references, given in Tables 7-9. All these mines are located at cement companies. Like the data analysis carried out for the Shahrood Cement Company, all the frequently-used PPV predictors listed in Table 3 were employed to obtain a valid and reliable predictor to estimate the peak particle velocity. The results of this analysis are given in Figure 9, and also the statistical summary of each predicting model are given in Table 10. Considering the results

tabulated in this table, the best model was found to be the USBM predictor with R square of 0.64 that is reliable to some extent. It can be seen that by using the additional recorded data, the estimation precision improved, and the R square of the best predictor was improved from 0.41 to 0.64, which is due to the extended database. In the next step, the general model would be used to estimate the PPV value in this extended database.

Table 7 Maa	anned with retion	data in Altaona	Comont Com	money limeston	
Table 7. Mea	sureu vidration	uala III Akcalisa	Сешені Сон	прану шпезионе	2 11111105 1421.
			0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0		

Row No.	PPV (mm/s)	Freq (Hz)	Total W (kg)	W (kg/delay)	<b>D</b> (m)
1	2.35	10	2102	538	257
2	1.2	10	1300	69	150
3	0.85	11	4390	81	255
4	0.95	5.6	3100	206	283
5	0.65	67	4165	93	295
6	0.70	91	3858	97	275
7	0.90	9.4	2225	175	254
8	144	20	180	180	23
9	0.25	9	4250	170	520
10	250	45	242	242	25.6
11	1.55	17	165	165	340
12	0.95	16	83	83	368
13	1.15	9.1	242	242	376
14	0.85	9.6	324	354	400
15	0.35	41	180	180	403
16	0.40	20	180	180	418

 Table 8. Measured vibration data in Assiut Cement Company limestone mines [2].

Row No.	<b>D</b> (m)	W (kg/delay)	Lon (mm/s)	Tran (mm/s)	Vert (mm/s)	PPV (mm/s)
1	595	820	1.5	1.2	1.2	2.26
2	729	820	1.2	1.7	1.2	2.40
3	875	820	1.5	2.0	2.0	3.20
4	901	830	0.7	0.7	1	1.41
5	876	830	1.0	1.2	2.7	3.12
6	587	1525	1.7	1.7	3	3.84
7	767	1525	2.2	1.7	1.2	3.03
8	823	1525	2.0	2.0	2.0	3.46
9	585	1115	1.5	1.7	3.3	4.00
10	565	1115	2.2	1.7	3.0	4.09

 Table 9.Measured vibration data in Egyptian Cement Company plant limestone mines [43].

Row No.	D (m)	W (kg/delay)	Long (mm/s)	Tran (mm/s)	Vert (mm/s)	PPV (mm/s)
1	544	661.5	1.4	2.23	0.9	2.78
2	770	975	0.93	1.33	0.7	1.77
3	778	874	1.73	2.29	1.5	3.24
4	538	400	1.51	1.71	1.44	2.70
5	864	535	1	1	1	1.73
6	536	164	1.18	1.22	1.12	2.03
7	732	330	1	1	1	1.73
8	608	378	1.34	1.98	1.5	2.82
9	824	367	1	1	1	1.73
10	494	228	1.2	1.5	1	2.17

Table 10. Statistical summary of PPV prediction models for all cement company data.

Predictor model	USBM	Langfors & Kihlstrom (1968)	Ambrases & Hendron (1968)	Indian standard
$R^2$	0.64	0.34	0.56	0.34
$k_1$	237.542	0.327	616.951	0.327
$k_2$	-1.537	1.954	-1.399	0.977



Figure 9. PPV prediction based on different predictors listed in Table 3 for extended database. a) USBM, b) Langfors & Kihlstrom (1968), c) Ambrases & Hendron (1968), and d) Indian standard.

# **5.1.** Analyzing general model using genetic algorithm (GA)

All the formulas listed in Table 3 are the simplified and summarized forms of the general formula that was proposed by Davies et al. [8], as follows:

$$PPV = k_1 . D^{-k_2} . W_{max}^{k_3}$$
(2)

In all the reviewed literatures, they used the formulas which included two independent constants  $k_1$  and  $k_2$ , describing the site specifications and characteristics. However, in this research work, there are three independent constants,  $k_1$ ,  $k_2$ , and  $k_3$ , describing the blast-site properties. In fact, in the summarized formulas, in order to simplify the relationship between the blast-induced PPV and D (the distance between the blast face and vibration monitoring point, m), W (the maximum charge per delay, kg), a hybrid

variable called SD (scaled distance) is defined, and in each relation, it has a pre-defined form, and so this form causes these relations not to establish a valid and reliable predictor for general situations. Each form is suitable for specific and particular conditions, and furthermore, even in an individual site, the two constants  $k_1$  and  $k_2$  in these relations cannot reflect the whole perspective of the geological and geotechnical conditions. To achieve a reliable predictor, the general forms of these predictors were employed, although finding such optimized values for the three constants involved  $(k_1, k_2, and k_3)$  in this relation could not be performed by a simple regression analysis. This problem was dissolved by the genetic algorithm (GA) optimization method, which was used to obtain the optimized constants  $k_1$ ,  $k_2$ , and  $k_3$ , as explained below.

### 5.2. Genetic algorithm optimization method

Optimization is the process of trying to find the best solution to a problem that may have many possible solutions. Most problems involve many variables that interact based on the given formulas and constraints.

Genetic algorithm (GA) is a global search technique, modeled after the process of natural selection, which can be used to find the near optimal solutions to the highly non-linear optimization problems [43].

The following outline summarizes how GA works:

1. The algorithm begins by creating a random initial population.

2. The algorithm then creates a sequence of new populations. At each step, the algorithm uses the individuals in the current generation to create the next population. To create the new population, the algorithm performs the following steps:

• Scores each member of the current population by computing its fitness value.

• Scales the raw fitness scores to convert them into a more usable range of values.

• Selects members, called parents, based on their fitness.

• Some of the individuals in the current population that have lower fitness are chosen as elite. These elite individuals are passed to the next population.

• Produces children from the parents. Children are produced either by making random changes to a single parent (mutation) or by combining the vector entries of a pair of parents (crossover).

• Replaces the current population with the children to form the next generation.

3. The algorithm stops when one of the stopping criteria (such as maximum iteration and time criterion) is met.

Using this optimization method, the purpose is to find the optimized values for the constants  $K_1$ ,  $K_2$ , and  $K_3$ , and then the optimized formula as  $PPV = K_1 \cdot D^{K_2} \cdot W^{K_3}$ . From the data measurement Tables 4, 7, 8, and 9, the real values for the recorded PPV or a set of vectors are available as below:

 $(PPV_i, W_i, D_i)$  i = 1 to n (3)

Where n is the number of measured records,  $W_i$ 

and  $D_i$  are the maximum charge per delay and the observation station distance from blast-site in each blasting sequence, respectively.

On the other hand, the PPV value can also be predicted using the general form including simultaneously the three constants  $K_1$ ,  $K_2$  and  $K_3$ , and obtaining the predicted PPV. The aim is to find the best constants  $K_1$ ,  $K_2$ , and  $K_3$ , so that the difference between the real and the predicted values for PPV should be minimized as much as possible by using the genetic algorithm optimization method. The predicted PPV values would be as:

$$PPV_j = K_1 . D_i^{K_2} . W_i^{K_3}$$
 i, j = 1 to n (4)

Where  $PPV_j$  is the predicted PPV value by Equation (2).

The optimization method was used to minimize the error between  $PPV_M$  (measured PPV) and  $PPV_C$  (calculated PPV using  $PPV_j = K_1 . D_i^{K_2} . W_i^{K_3}$ ), and consequently, to increase the correlation between them, as follows:  $EPPOP = (PPV - PPV)^2$  (5)

$$ERROR_{k} = (PPV_{j} - PPV_{i})^{2}$$
(5)

$$Total \ error = \sum_{i=1}^{n} \left( k_1 . D_i^{k_2} . W_i^{k_3} - PPV_i \right)^2$$
(6)

In fact, in the genetic model, the fitness function is the total error given by Equation (6), which should be minimized, and so it can be written as follows:

Fitness Function = 
$$\sum_{i=1}^{n} (k_1 \cdot D_i^{k_2} \cdot W_i^{k_3} - PPV_i)^2$$
 (7)

In order to use the GA optimization method, in this work, the MATLAB program was used.

After optimizing with the GA, the optimized values were achieved for the constants, as follow:

 $k_1 = 5028.891, k_2 = -1.81523, k_3 = 0.524601,$ 

and the final equation would be as follows: 1.81 - 0.52

$$PPV = 5028.89D^{-1.81}.W_{\rm max}^{0.52} \tag{8}$$

To evaluate the correlation level between the measured and calculated peak particle velocities, a linear regression was fitted (Figure 10). It could obviously be seen in this figure that the two data sets are close enough to each other. In fact, the slope of the fitted line was1.1, which can be rounded to 1, and its equation was given as Equation (9). After all, by considering a satisfactory engineering precision, this model is the best predictor of all. The statistical summary of this model was summarized in Table 11. The equation for the linear fitted curve is as follows:

$$PPV_C = 1.1 \times PPV_M \qquad R = 0.92 \qquad (9)$$

Where  $PPV_M$  and  $PPV_C$  are the measured and calculated values of peak particle velocities, respectively.

In a blasting design, there are several criteria that should be considered such as powder factor (specific charge) and fly rock. The maximum allowable PPV level is one of them, and this restrictor parameter controls and dictates the maximum amount of charge per delay for blast designers to prevent from excessive vibration occurrence. For this aim, the diagram of W (Maximum amount of charge per delay) versus D (distance) including different PPV levels was drawn (Figure 11).

As it can be seen in this figure, for any given distance from the blasting site and an allowable PPV, the amount of charge per delay can be estimated and determined using the curve of charge per delay *vs*. distance from blasting site.



Figure 10. Correlation between PPV calculated by general formula and measured PPV.

Table 11. Statistical summary of PPV prediction.



Figure11. Prediction of maximum charge per delay based on based acceptable level of PPV and distance form blast site (A=200 mm/s, B=100mm/s, C=50mm/s, D=30mm/s, E=10mm/s, F=5mm/s, G= 3mm/s, H=1 mm/s, and I=0.5 mm/s).

# 6. Conclusions

In this research work, the general aim was to propose a general blast-induced vibration predicting model for limestone mines. The work was focused on field observation and surveying the Shahrood limestone mines. After measuring the required data and parameters, the field measurement results were analyzed and interpreted using the frequently-used peak particle velocity predictors. Then the best predictor was chosen to estimate the PPV level in these mines. In this analysis, the best models were Langfors & Kihlstrom and Indian standard predictors with an R square of 0.41. In order to generalize the relationships between different limestone mines, and also to increase the prediction precision, some data related to similar limestone mine studies were gathered from several references, and then analyzed using the same predictors. The best predictor model for this stage was found to be the USBM predictor with an R square of 0.64. By adding additional databases, the prediction improvement was found to be significant.

Ultimately, the general PPV predication model was applied to the extended database, and then using the genetic algorithm (GA) optimization method, the constants of the model describing the geotechnical and other blast properties were obtained. The correlation between the values resulting from this model and the measured ones was 0.92, which is satisfactory and reliable.

## Acknowledgments

The authors wish to express their gratitude to the research council of Shahrood University of Technology. We are also thankful to the officials of the Shahrood Cement Company for providing the necessary facilities during the course of investigation.

Special thanks go to N. Ziyari, A. Mirzai, and A. Mortazavi for their assistance during the field observations.

## References

[1]. Ak, H., Iphar, M., Yavuz, M. and Konuk, A. (2009). Evaluation of ground vibration effect of blasting operations in a magnesite mine. Soil Dynamics and Earthquake Engineering. 29: 669-676.

[2]. Tantawy, M.M. (2009). Artificial neural network for prediction and control of blasting vibrations in Assiut (Egypt) limestone quarry. International Journal of Rock Mechanics & Mining Sciences. 46: 426-431.

[3]. Basu, D. and Sen, M. (2005). Blast induced ground vibration norms- A critical review. National Seminar on Policies. Statutes & Legislation in Mines.

[4]. Duvall, W.I. and Fogelson, D.E. (1962). Review of criteria for estimating damage to residences from blasting vibration.US Bureau of Mines R.I. 5968.

[5]. Ambraseys, N.R. and Hendron, A.J. (1968). Dynamic behavior of rock mass. In: Proceedings of the rock mechanics in engineering practices. London. 203-207.

[6]. Langefors, U. and Kihlstrom, B. (1978). The modern technique of rock blasting. New York: John Wiley.

[7]. Ghosh, A. and Daemen, J.K. (1983). A simple new blast vibration predictor of ground vibrations induced predictor. In: Proceedings of the 24<sup>th</sup> US symposium on rock mechanics. Texas.

[8]. Davies, B., Farmer, I.W. and Attewell, P.B. (1964). Ground vibrations from shallow sub-surface blasts. The Engineer. London. 553-559.

[9]. Singh, T.N., Amit, P., Saurabh, P. and Singh, P.K. (2002). Prediction of explosive charge for efficient mining operation. In: Proceedings of the rock engineering-problems and approaches in underground construction. Seoul. 22-24 July 2002.

[10]. Ataei, M. (2010). Evaluation of Blast-Induced ground vibrations from underground excavation at Karoun 3 area, Mining Technology: Transactions of the Institute of Mining & Metallurgy. Section A. 119 (1): 7-13.

[11]. Kamali, M. and Ataei, M. (2010). Prediction of blast induced ground vibrations in Karoun III power plant and dam: a neural network. Journal of the South African institute of mining and metallurgy (SAIMM). 110: 1-10.

[12]. Sereshki, F., Ataei, M. and Kamali, M. (2010). Environmental concerns of blasting projects in limestone mines. Int. J. Mining and Mineral Engineering. 2 (4): 349-369.

[13]. Kamali, M. and Ataei, M. (2011). Prediction of blast induced vibrations in the structures of Karoun III Power Plant and Dam. Journal of Vibration and Control. 17 (4): 541-548.

[14]. Ghasemi, E., Ataei, M. and Hashemolhosseini, H. (2013). Development of a fuzzy model for predicting ground vibration caused by rock blasting in surface mining. Journal of Vibration and Control. 19 (5): 755-770.

[15]. Ataei, M. and Kamali, M. (2013). Prediction of blast induced vibration by adaptive neuro fuzzy inference system in Karoun 3 power plant and dam, Journal of Vibration and Control. 19 (12): 1906-1914.

[16]. Thoenen, J.R. and Windes, S.L. (1942). Seismic Effects of Quarry Blasting. U.S. Bureau of Mines Bulletin 442.

[17]. Kahriman, A. (2001). Analysis of Ground Vibrations Caused by Bench Blasting at Can Open-pit Lignite Mine in Turkey. Environmental Geology. 41: 653-661.

[18]. Crandell, F.J. (1949). Ground vibration due to blasting and its effect upon structures. Journal of Boston society of civil engineers.

[19]. Langefors, U., Kihlstrom, B. and Westerberg, H. (1957). Ground vibration in blasting. Water power.

[20]. Edwards, A.T. and Northwood, T.D. (1959). Experimental studies of the effects of blasting on structures. The Engineer 210: 538-546.

[21]. Nicholls, H.R., Johnson, C.F. and Duvall, W.I. (1971). Blasting vibration effects on structures. US Bureau of Mines report of investigation 656.

[22]. Indian Standard Institute. (1973). Criteria for safety and design of structures subjected to underground blast. ISI Bull 1973. IS-6922.

[23]. Medearis, K. (1976). The Development of Rational Damage Criteria for Low-Rise Structures Subjected to Blasting Vibrations. Report to the National Crushed Stone Association, Washington, D.C.

[24]. Canmet, B.A. and Calder, P.N. (1977). Pit slope manual. Chapter 7. Canmet report 77-14.

[25]. Siskind, D.E., Stagg, M.S. and Kopp, J.W. (1980). Dowding CH. Structure response and damage produced by ground vibration from surface mine blasting. US Bureau of Mines Report of Investigation 8507.

[26]. New, B.M. (1986). Ground vibration caused by civil engineering works. Transport and Road Research Laboratory, Research Report 53. 19 P.

[27]. CMRI. (1993). Vibration standards. Central Mining Research Institute. Dhanbad.

[28]. Rosenthal, M.F. and Morlock, G.L. (1987). Blasting guidance manual. U.S Office of surface mining reclamation and enforcement.

[29]. Burrus, C.S. and Parks, T.W. (1985). DFT/FFT and convolution algorithms. Rice University: John Wiley and Sons.

[30]. Light, H.M.J. (1980). An introduction to Fourier analysis and generalized functions. Cambridge. England: Cambridge University Press.

[31]. Oppenheim, A.V. and Schafer, R.W. (1975). Digital signal processing. Englewood Cliffs, NJ: Prentice-Hall, Inc.

[32]. Rabiner, L.R. and Gold, B. (1975). Theory and application of digital signal processing. Englewood Cliffs, NJ: Prentice-Hall, Inc.

[33]. Brigham, E.O. (1974). The fast Fourier transform. Englewood Cliffs, NJ: Prentice-Hall, Inc.

[34]. Singh, P.K. and Roy, M.P. (2008). Damage to surface structures due to underground coal mine blasting: apprehension or real cause?. Environ Geol. 53: 1201-1211.

[35]. Singh, P.K., Vogt, W., Singh, R.B., Singh, M.M. and Singh, D.P. (1997). Response of surface structures to rock blasting. Mineral Resource Engineering. 6 (4): 185-194.

[36]. Kuzmenko, A.A., Vorobey, V.D., Denisyuk, I.I. and Dauetas, A.A. (1993). Seismic Effects of Blasting in Rock. Balkema, Rotterdam. pp. 16-31.

[37]. Just, G.D. and Chitombo, G.P. (1987). The economic and operational implications of blast vibration limit mining and environment. The Aus. IMM. Australia. pp. 117-124.

[38]. Persson, P.A., Lundborg, N. and Johansson, C.H. (1980). The basic mechanisms in rock blasting, Proc. 2<sup>nd</sup> Cong. Rock mech. ISRM. Belgrade. Yugoslavia III. pp. 19-33.

[39]. Dhar, B.B., Pal, R.P. and Singh, R.B. (1993). Optimum blasting for Indian geo-mining conditionssuggestive standard and guidelines. CMRI Publication. India. 40 P.

[40]. Pal, R.P. (2005). Rock Blasting Effects and Operations. Balkema Publishers. 345 P.

[41] Kahriman, A. (2004). Analysis of parameters of ground vibration produced from bench blasting at a limestone quarry. Soil Dynamics and Earthquake Engineering. 24: 887-892.

[42]. Elseman, I. and Rasoul, A. (2000). Measurement and analysis of the effect of ground vibrations induced by blasting at the limestone quarries of the Egyptian Cement company. I.A. Elseman/ ICEHM 2000. Cairo University. Egypt. pp. 54-71.

[43]. Cheng, C.T., Zhao, M.Y., Chau, K.W. and Wu, X.Y. (2006). Using genetic algorithm and TOPSIS for Xinanjiang model calibration with a single procedure. Journal of Hydrology. 316: 129-140.

# بهبود پیشبینی لرزشهای ناشی از انفجار در معادن سنگ آهک با استفاده از الگوریتم ژنتیک

محمد عطائی ؓ و فرهنگ سرشکی

دانشکده مهندسی معدن، نفت و ژئوفیزیک، دانشگاه صنعتی شاهرود، ایران

ارسال ۲۰۱۶/۵/۲۲، پذیرش ۲۰۱۶/۶/۲۷

\* نویسنده مسئول مکاتبات: ataei@shahroodut.ac.ir

#### چکیدہ:

مشابه بسیاری از معادن سنگ آهک که مواد خام مورد نیاز کارخانه سیمان را تأمین میکند، هزینه انتقال مواد خام در کارخانه سیمان شاهرود بالا است. تلاش شده است که واحد سنگ شکن و آسیا در نزدیک ترین فاصله ممکن از معدن ایجاد شود. از سوی دیگر، انفجار اثرات منفی داشته و تخریب های ناشی از انفجار بر روی ساختمان ها و ماشین آلات قابل توجه است. در چنین معادنی از بین اثرات انفجار، لرزش های ناشی از انفجار مار گرفته و یک رابطه معتبر برای پیش بینی اثرات لرزش های ناشی از انفجار در چنین مناطقی به ویژه شرکت سیمان شاهرود ارائه شده است. مورد تحلیل قرار گرفته و یک رابطه معتبر برای پیش بینی اثرات لرزش های ناشی از انفجار در چنین مناطقی به ویژه شرکت سیمان شاهرود ارائه شده است. تاکنون شاخصهای مختلفی برای ارزیابی اثرات لرزش های ناشی از انفجار در چنین مناطقی به ویژه شرکت سیمان شاهرود ارائه شده است. تاکنون شاخصهای مختلفی برای ارزیابی اثرات لرزش های ناشی از انفجار در چنین مناطقی به ویژه شرکت سیمان شاهرود ارائه شده است. تاکنون شاخصهای مورد توجه قرار گرفته است. اول اکثر سازی از تان (PPV) توسط بسیاری از محقین شاون شاخصهای مغربی ماز مان ارزیابی اثرات لرزش های ناشی از انفجار معرفی شده است که در این حین حداکثر سرعت ذرات (PPV) توسط بسیاری از مدو شاخ مورد توجه قرار گرفته است. به منظور ایجاد رابطه بین PPV و ویژگیهای منطقه انفجار، تاکنون روابط مختلفی ارائه شده است. قالب اکثر روابط ارائه شده بصورت شاخ گرفته است. برای شرخ این رابطه W و D به ترتیب حداکثر خرج در هر تراخیر و فاصله از محل انفجار و K<sub>1</sub> K<sub>2</sub> که ی منطقه را تشریح می کنند. در این تحقیق مجموعهای از آزمایش ها و برداشتهای صحرایی انجام و پارامترهای لازم جمعآوری شده است. سخس به منظور قابلیت تعمیم رابطه رائه شده و همچنین افزایش دقت، داده های معادن سنگ آهک مشابه نیز جمعآوری شده است. بو سیم می مان را قار مان مده است. مورد و به منظور یافتن بهترین رابطه براز می منظور قابلیت تعمیم رابطه ارائه شده و الغوریتم ژنتیک مورد استاه هرد سنگ آهک مشابه نیز جمعآوری شده است. به منظور یافتن بهترین رابطه براز می منظور قابلیت تعمیم رابطه ارائه شده و الگوریتم ژنتیک مورد استفاده قرار گرفته است.

كلمات كليدى: انفجار، لرزش ناشى از انفجار، PPV، معدن سنك آهك، كارخانه سيمان، الكوريتم ژنتيك.